




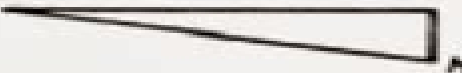


















Table 3.1 — Values of K for various bending moment diagrams

Loading	Bending moment diagram	K
		0.125
		$\frac{3 - 4a^2}{48(1-a)}$ if $a = \frac{1}{2}$ $K = \frac{1}{12}$
		0.0625
		$0.125 - \frac{a^2}{6}$
		0.104
		0.102
		$K = 0.104 (1 - \frac{\beta}{10})$ $\beta = \frac{M_A + M_B}{M_C}$
		end deflection $= \frac{a(3-a)}{6}$ load at end $K = 0.333$
		$\frac{a(4-a)}{12}$ if $a = l$ $K = 0.25$
		$K = 0.083 (1 - \frac{\beta}{4})$ $\beta = \frac{M_A + M_B}{M_C}$
		$\frac{1}{80} \frac{(5-4a^2)^2}{3-4a^2}$

Steel Manual Deflection Equations

Daniel Kinnear Clark



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